The 'Bell-Building': A new type of earthquake resistant structure

H. Wittfoht & G. Cretu H. Wilder, Bad Homburg, V.D.H., FR Germany Pollansky & Zollner, Bad Homburg, V.D.H., FR Germany

ABSTRACT: This paper presents an alternative structural concept for mul-ABSTRACT buildings, especially when an open first floor is required. The improved seismic performance makes possible the construction of such a improved even in environments where strong earthquakes have to be taken building even in Duo to the strong earthquakes have to be taken bull consideration. Due to the boundary conditions of different structural components, the bell will practically remain in vertical position, even during an earthquake. Because the "Bell Building" strucpositive remains in elastic range under all loading combinations, including strong earthquakes, the self-centering will never be a problem.

1 INTRODUCTION

The search for appropriate earthquake resistant structures has been underway since buildings have been

damaged in earthquakes.

The progress in this field of activity was quite slow, from masonry construction in 1900 to steel and reinforced concrete framing in the 1920's, to welded steel frame assemblies in the 1950's, to ductile concrete frames in the 1970's, to eccentric braced steel frames in the 1980's. We are now beginning to design and build with new concepts using isolation and passive energy dissipation devices.

These new concepts are very useful, especially in the design of buildings whose fundamental frequency of vibration is in the range of frequencies where earthquake energy is strongest. By means of adequate isolation and passive energy dissipation devices, the frequency of the structure can be significantly altered. This clearly results in a significant response reduction for most sites.

Such a structure as a whole is more flexible than a typical one. For this reason it should be emphasized that softening a structure can mean an invitation to higher or

lower inertia forces, depending on its relation to the frequency content of the ground motion. It is self-evident that long-period ground motions, defined herein as motions with periods of more than one second, can therefore be an important issue in the design and performance of structures incorporating isolation and/or energy dissipation.

The goal of current codes is to prevent structural collapse and

loss of lives.

Isolation, respectively baseisolation, and passive energy dissipation devices are intended to provide a level of structural performance that goes well beyond the normal code requirements. In this case, however, it should be kept in mind that new systems generate new problems. Whenever a new type of structure has to be designed and an earthquake hazard must be taken into account, it should never be forgotten that each earthquake provides many surprises in structural performance which are not easily reconciled with existing theories of behavior. Accordingly, the design of structures incorporating isolation and/or passive energy dissipation systems is a complex mat-

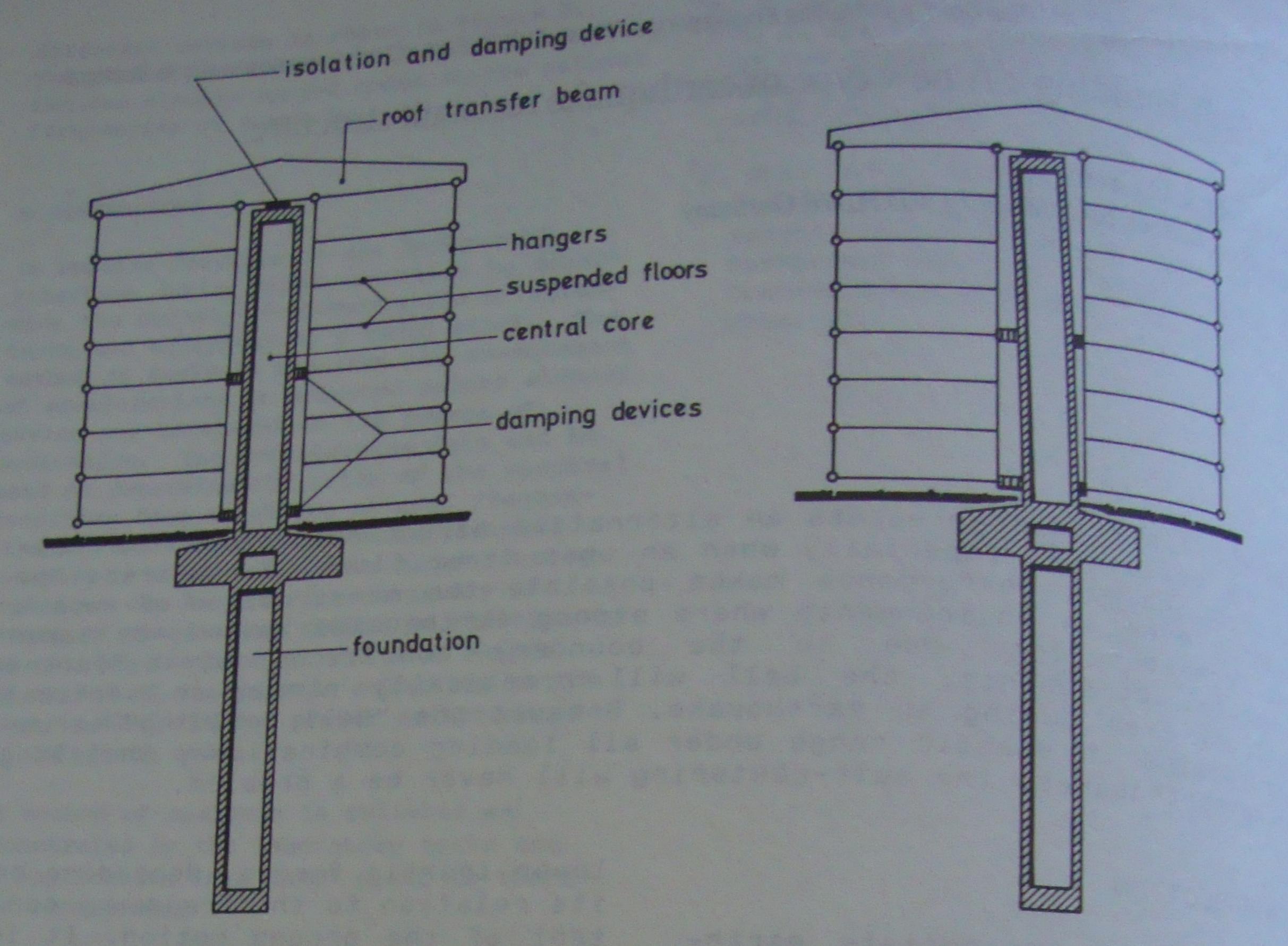


Fig. 1 Schematic cross-section of the "Bell Building".

Fig. 2 Distorted geometry of the "Bell Building" under earthquake loading combination.

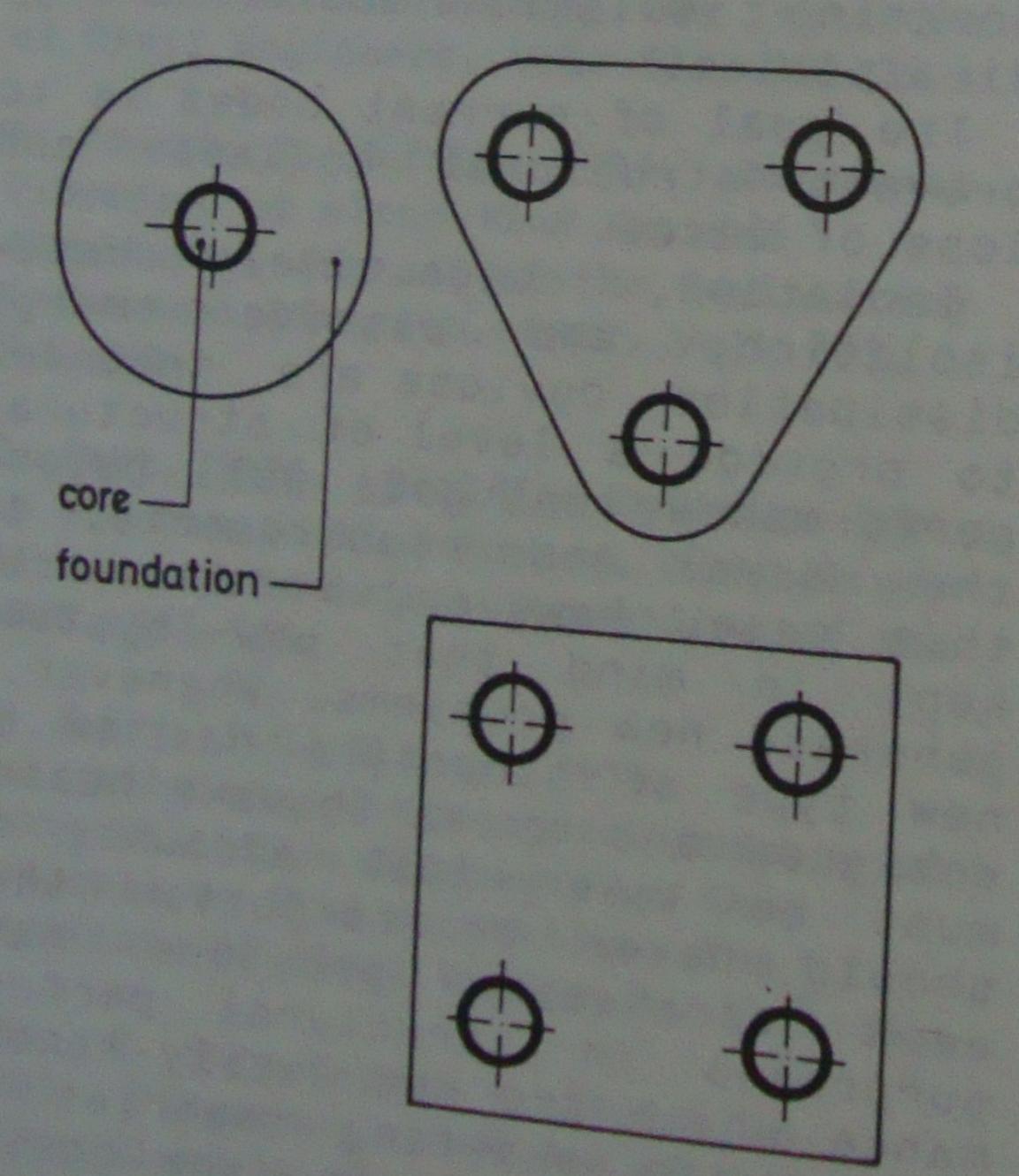


Fig. 3 Possible arrangement of the cores in a "Bell-Building".

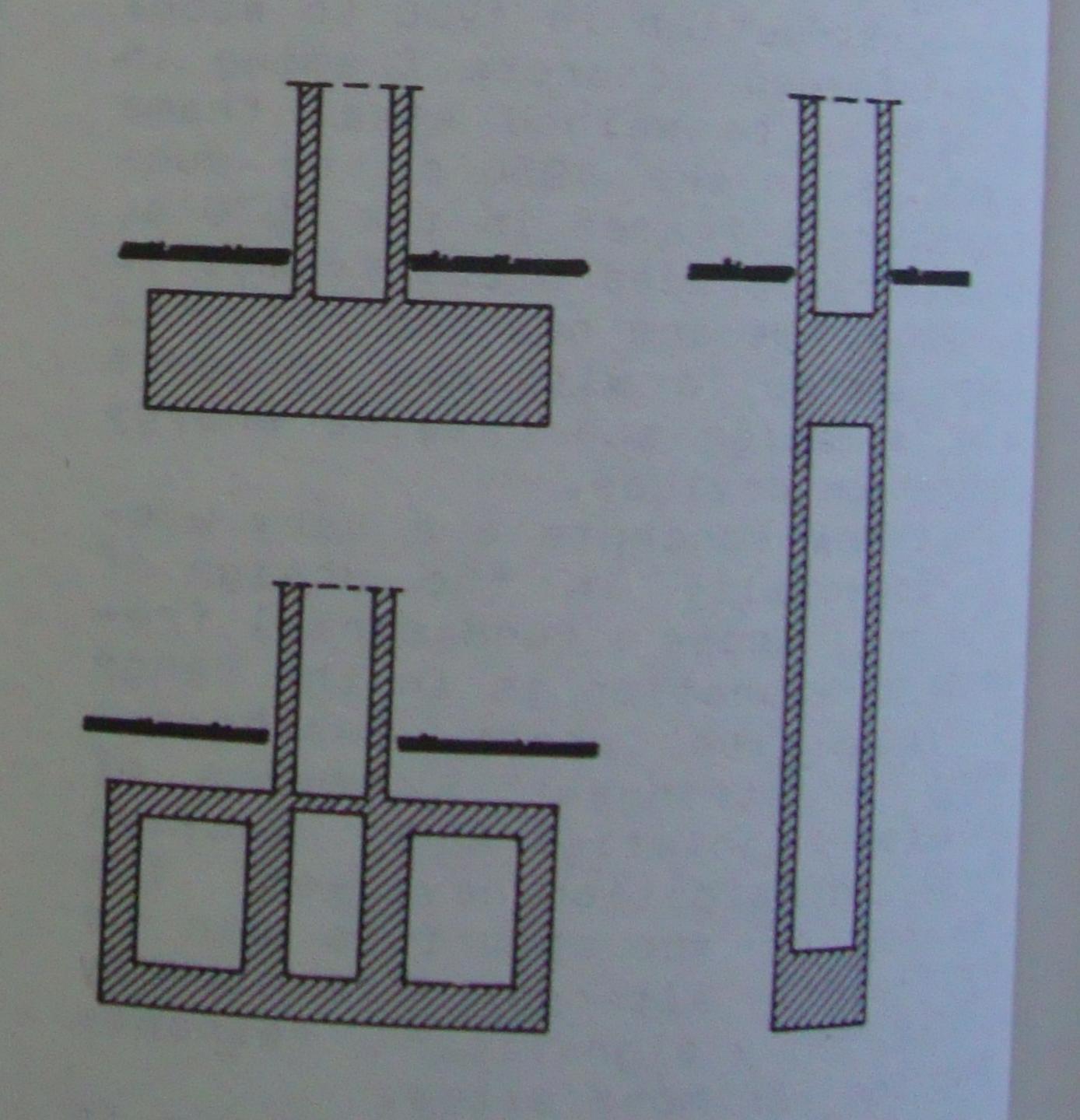


Fig. 4 Different types of foundations for a "Bell-Building".

requiring complex design and ter requiring techniques, including the analysis techniques, including the analysis computers and sophisticated use of computers and sophisticated use of tware. It is this complexity software at the root of research on, that is at the root of research on, and practice of, earthquake engineering.

2 THE BELL BUILDING

2.1 General considerations

particularly in the earthquake engineering field of activity, design professions are likely to agree to innovations that do not agree to innovations that do not represent a large departure from current practice. This suggests that the most readily accepted innovative systems will be those innovative systems will be those that improve the properties and performances of a typical structure. The "Bell Building" is a typical structure, but with improved seismic performance.

Before describing the above-mentioned structure, it is advisable to review the past and thus to account for the reasons which have prompted the study of the "Bell

Building".

In the past as in the present, the discrepancy between the nature of the buildings that engineers study and those which architects design and build may be clearly noticed. The engineer conceives a building in engineering terms, and the architect, on the other hand, sees engineering profession as a means of achieving buildings which have arisen from different criteria, such as, originality, uniqueness, image, etc.

For this reason, perhaps, the "soft first story" concept continues to be taken into consideration, even if its seismic performances

are by far not the best.

The aesthetic attraction of the "floating box" with its open first floor is very real to architects and urban designers interested in plazas that link the building to the street. The high first floor is very useful when large spaces such as banking floors - have to be provided at ground floor level. Therefore, it seems assured that the "floating box" will be with us in the foreseeable future.

According to the above-mentioned considerations, the following questions need an explicit answer: Is it possible to erect in a seismic environment a reliable and economical building having an open first floor with no restriction concerning the floor's height or inner distribution, which concurrently avoids the poor seismic performance of the first soft story? Is it possible to reconcile the otherwise conflicting aims of architects and engineers? Yes, it is.

The building satisfying the above-mentioned requirements is the "Bell Building".

2.2 General description

The structural components of such a building are (fig. 1):

- The foundation which can be of different types depending on the soil report recommendations

(fig. 4).

- The central core, or cores, bearing the loads of the bell.

- The "Bell", consisting of floors plus vertical structural and non-structural components, which transfer the loads at the top of the core by means of special isolation and damping devices.

- Springs and damping devices placed between the different floors and the core, with the aim of controlling the relative displacements between the floors and the core.

The allowable relative displacements between the core and the floors depend on a lot of parameters and are generally specified in the "technical requirements of performance" drawn-up by the owner and its consultant or in the earthquake regulations of the country where the structure has to be built.

The allowable relative displacements between the floors and the ments between the floors and the core as well as the data concerning the ground motion are of ning the ground motion are of essential importance for the dimensioning of the springs, isolamensioning of the springs, isolation, and damping devices.

2.3 Behavior and performances

Under normal operating loads, i.e. vertical loads, wind forces or minor earthquakes, the behavior of the "Bell Building" is similar to that of a normal suspended structure without isolation and damping devices. Certainly, all these devices placed in a "Bell Building" must have adequate constants so that the normal operating loads may be carried without excessive displacements or unpleasant oscil-

Under the loading combination of lations. vertical loads and inertia forces due to a strong ground motion (fig. 2), the core will deflect but the bell will remain practically vertical. The special springs and damping devices placed between the floors and the core avoid any excessive relative displacements between the two structural components as well as un-

pleasant oscillations.

Because all vertical loads of the suspended floors are transferred at the top of the core, i.e. above the "bell's" center of gravity, a stable state of equilibrium is always ensured. The stiffness of the core, constant from top to bottom, the concentration of the vertical loads at the top of the core, and the isolation and dampening devices placed between the core and the bell will reduce the accelerations in the structure and the inertia forces to the advantage of the occupants and the contents of the building. The period of the fundamental mode may be increased well beyond 2 sec., making this type of structure suitable for most sites.

The proposed structure may be partially or fully isolated, depending on the direction and intensity of the earthquake acceleration. The new structural concept not only allows the building to remain elastic during a strong earthquake, but spares the nonstructural elements from extensive distress. Since the non-structural components in a typical multistory structure account for about 50% - 80% of the building's cost, a positive means of preventing distress in such elements during strong earthquakes would represent

significant savings in the and replacement costs which repair would arrange be required.

since the proposed building will elastic during a stro since the property during a long will remain elastic during a will earthquake, the self-centering of automat. the structure will occur automatic

The proposed building can hieved as a base isolate the proposition as a base an also be achieved as a base isolated the safet suspended structure, if the safed and the safety against overturning and the safety against after an earthquake against over an earthquake self not a problem, and the mainte is inspection, and replacem nance, inspection, and replacement may of base isolation devices may be

3 CONCLUSIONS

The "Bell Building" alternative structural concept for provides an multistory buildings, especially when an open first floor is

The proposed concept allows the construction of the open first floor even if a strong ground motion has to be taken into account, removing the poor seismic performance of the soft first story concept.

The new alternative is also an attempt to reconcile the otherwise conflicting aims of architects and engineers, and to change the open first floor structural concept from a liability to an asset.

REFERENCES

Arnold, C., Reithermann, R. 1982. Building configuration and seismic design. New York: John Wiley & Sons.

Arnold, C. 1984. Soft first stories: truths and myths. Proc. Eighth World Conference on Earthquake Engineering. San Francisco, California, U.S.A.

Boardman, P.R., Wood, B.J. and Carr, A.J. 1983. Union House A braced structure with energy dissipators. Bulletin of the New Zealand National Society for Earthquake Engineering. Vol. 16, No. 2, p.83-97.

Caspe. M.S. 1970. Earthquake isolation of multistory concrete structures. Journal of the American Concrete Institute.

American Concrete Institute. No. 11, p. 923-933. Chopra, A.K., Clough, D.P. and Clough, R.W. 1973, Earthquake resistance of buildings with a soft first story. International Journal of Earthquake Engineering and Structural Dynamics. Vol. 1, p. 347-355. Clough, R.W., Penzien, J. 1982. Dynamics of structures. London: Mc Graw-Hill International Book Company. Fintel, M., Kahn, F.R. 1969, Shock absorbing soft-story concept for ACI Journal, Proc. 66 (5).

multistory earthquake structures. p.381-390.

Ikonomou, A.S. 1984. Alexisismon seismic isolation levels for translational and rotational seismic input. Proc. Eighth World Conference on Earthquake Engineering. San Francisco, California, U.S.A.

Kelly, J.M. 1979. Aseismic base isolation: a review. Proc. of the 2nd U.S. National Conference on Earthquake Engineering, August 22-24, Standford University, Standford, California, U.S.A.

Newmark, N.M. 1969. Torsion in symmetrical buildings. 4th World Conference on Earthquake Enginerring. Santiago, Chile

Novak, M., El Hifnawy, L. 1983. Effect of soil-structure interaction on damping of structures. Earthquake Engineering and Structural Dynamics. Vol. 11. Rosenblueth, E. 1980. Design of

earthquake resistant structures. London: Pentech Press.